## AREA 20 PRECINCT - DRAINAGE RESERVE No. 1 (REFER FIGURE 4)

## **CHANNEL FLOW - MANNINGS**

#### Q=(AR^0.6667S^0.5)/n

Q (m^3/s) 6.950 100 Yr flow S (%) 1.000 0.030 OFFSET LEVEL Base Width (m) 6.000 -8.774 0.992 Base Crossfall (%) 1.000 -3.000 0.030 6.000 Top Width (m) 17.547 0.000 0.000 Batter Slope (1 in) 6.000 3.000 0.030 Max Depth (m) 1.000 8.774 0.992 6.000 Max Velocity (m/s) 2.000

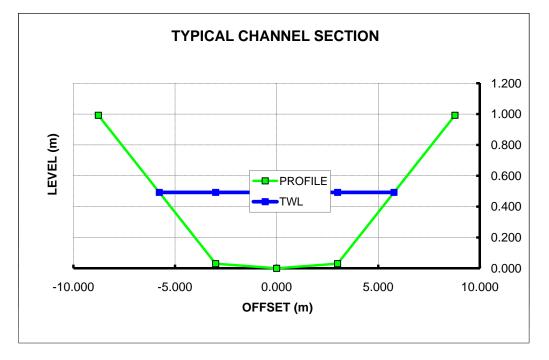
0.500

ABA0 667 2 005

Freeboard (m)

**AR^0.667** 2.085 **Excavation (sq.m)** 11.419

Flow Type	DEPTH	Α	Р	R	AR^0.67	WIDTH	٧	FBRD	F	V*D
	(m)	(m^2)	(m)	(m)		(m)	(m/s)	(m)		
Normal	0.492	4.146	11.624	0.357	2.085	11.547	1.676	0.500	0.893	0.825
Critical	0.460	3.781	11.233	0.337	1.830	11.162	1.838	0.532	1.008	0.846



## AREA 20 PRECINCT - DRAINAGE RESERVE No. 2 (REFER FIGURE 4)

16.220

## **CHANNEL FLOW - MANNINGS**

#### Q=(AR^0.6667S^0.5)/n

Q (m<sup>3</sup>/s) 17.900 100 Yr flow

**S (%)** 1.000

0.030 OFFSET LEVEL Base Width (m) 22.000 -16.220 0.980 Base Crossfall (%) 1.000 -11.000 6.000 0.110 Top Width (m) 32.439 0.000 0.000 Batter Slope (1 in) 11.000 0.110 6.000

1.000

Max Velocity (m/s) 2.000 Freeboard (m) 0.500

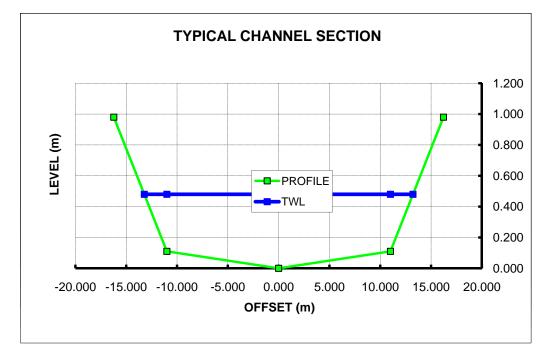
Max Depth (m)

**AR^0.667** 5.370 **Excavation (sq.m)** 24.889

Flow Type	DEPTH	Α	Р	R	AR^0.67	WIDTH	٧	FBRD	F	V*D
	(m)	(m^2)	(m)	(m)		(m)	(m/s)	(m)		
Normal	0.480	10.169	26.501	0.384	5.370	26.439	1.760	0.500	0.906	0.845
Critical	0.454	9.485	26.185	0.362	4.820	26.127	1.887	0.526	1.000	0.857

0.980

6.000



## **Attachment G**

Drainage Reserves / Easements Hydraulic Calculations (Including the Impact of Climate Change)

## AREA 20 PRECINCT - DRAINAGE RESERVE No. 1 (REFER FIGURE 4) INCLUDING CLIMATE CHANGE

### **CHANNEL FLOW - MANNINGS**

#### Q=(AR^0.6667S^0.5)/n

Q (m<sup>3</sup>/s) 8.630 100 Yr flow

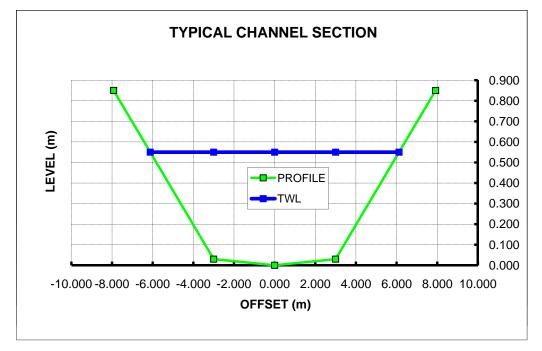
**S (%)** 1.000

0.030 OFFSET LEVEL Base Width (m) 6.000 -7.920 0.850 Base Crossfall (%) 1.000 -3.000 0.030 6.000 Top Width (m) 15.841 0.000 0.000 Batter Slope (1 in) 6.000 3.000 0.030 Max Depth (m) 1.000 7.920 0.850 6.000

Max Velocity (m/s) 2.000 Freeboard (m) 0.300

**AR^0.667** 2.589 **Excavation (sq.m)** 9.045

Flow Type	DEPTH	Α	Р	R	AR^0.67	WIDTH	V	FBRD	F	V*D
	(m)	(m^2)	(m)	(m)		(m)	(m/s)	(m)		
Normal	0.550	4.833	12.327	0.392	2.589	12.241	1.786	0.300	0.907	0.982
Critical	0.460	3.781	11.233	0.337	1.830	11.162	2.282	0.390	1.252	1.050



## AREA 20 PRECINCT - DRAINAGE RESERVE No. 2 (REFER FIGURE 4) INCLUDING CLIMATE CHANGE

### **CHANNEL FLOW - MANNINGS**

#### Q=(AR^0.6667S^0.5)/n

Q (m<sup>3</sup>/s) 20.800 100 Yr flow

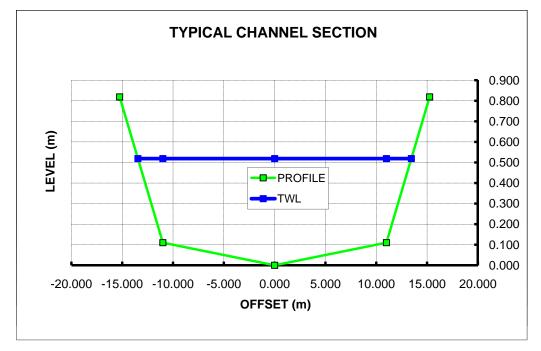
S (%) 1.000

0.030 OFFSET LEVEL Base Width (m) 22.000 -15.253 0.819 Base Crossfall (%) 1.000 -11.000 0.110 6.000 Top Width (m) 30.506 0.000 0.000 Batter Slope (1 in) 6.000 11.000 0.110 Max Depth (m) 1.000 15.253 0.819 6.000

Max Velocity (m/s) 2.000 Freeboard (m) 0.300

**AR^0.667** 6.240 **Excavation (sq.m)** 19.819

Flow Type	DEPTH	Α	Р	R	AR^0.67	WIDTH	٧	FBRD	F	V*D
	(m)	(m^2)	(m)	(m)		(m)	(m/s)	(m)		
Normal	0.519	11.207	26.975	0.415	6.240	26.906	1.856	0.300	0.918	0.963
Critical	0.494	10.548	26.675	0.395	5.682	26.610	1.972	0.325	1.000	0.975



# Attachment H Flood Safety Evaluation



15 June 2010

Mr Daniel Gardiner Water Resources Engineer J. Wyndham Prince PO Box 4366 PENRITH WESTFIELD 2750

Dear Daniel

### Re: Area 20 Rouse Hill - Flood Safety Evaluation

Thankyou for sending through the latest flood extent and subdivision layout for Area 20 at Rouse Hill.

As requested we have analysed the information and formed the following views with regard to flood safety aspects of the proposed development.

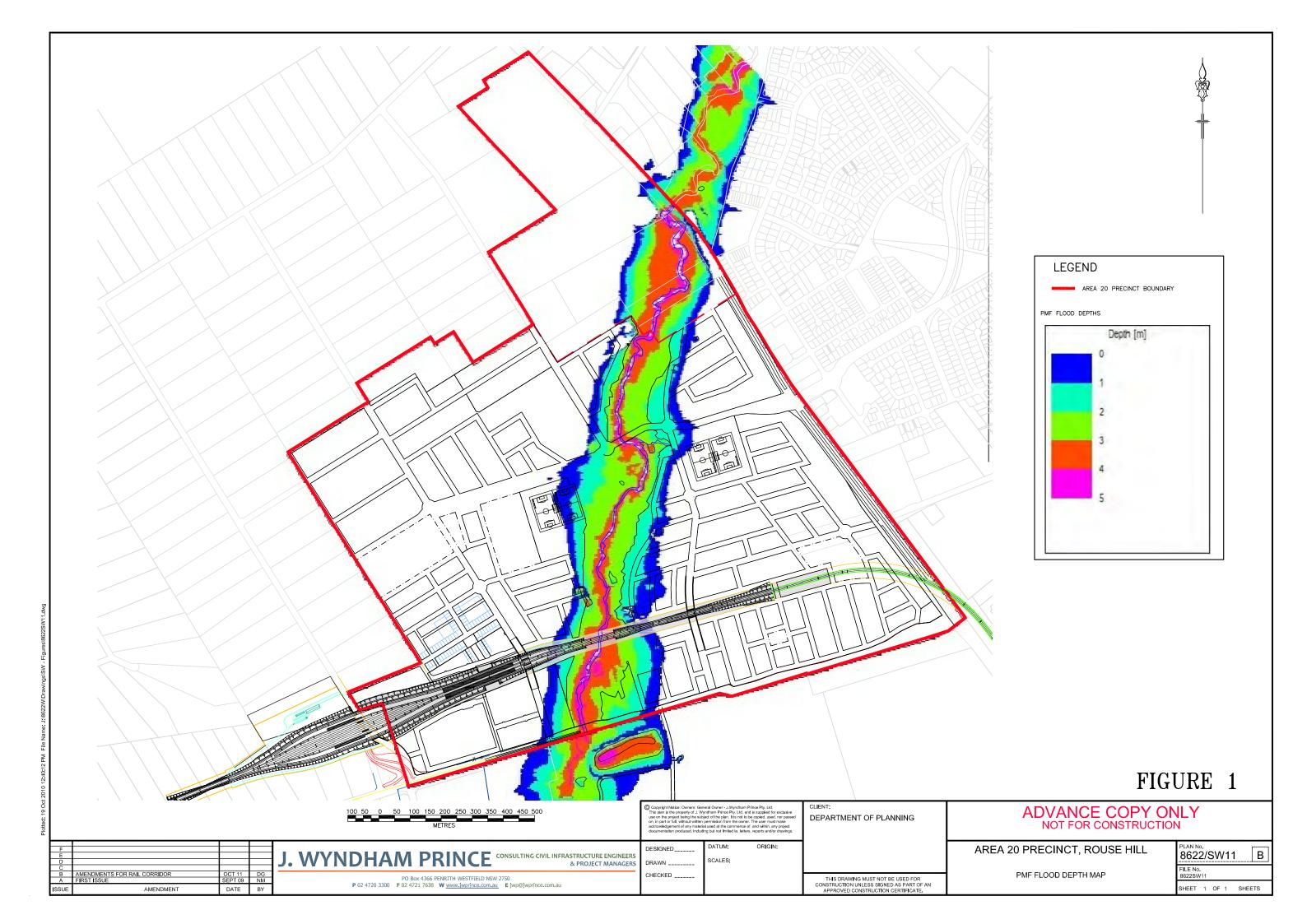
- 1. The development is all above 42m AHD which is well above the 26m AHD probable maximum flood (PMF) level on the Hawkesbury Nepean River. The development is therefore outside of any influence from backwater flooding from the Hawkesbury Nepean River. Flooding from the River is therefore not an issue with regard to safety in the development;
- 2. All residential development will be above the 1 in 500 flood level from the local creek so there is a low probability that any homes will get flooded;
- 3. The creek PMF does not extend very far into the residential areas and is no more than 2m deep on any residential land. Even such an event is likely to pose a low risk to life for people in residential dwellings who should be able to shelter in place because:
  - a. the ground floor level of all homes will be at least 300mm above ground level:
  - b. most of the homes that will be built will be double storey and;
  - c. the duration of PMF flooding within homes is likely to be less than an hour;
- 4. The proposed subdivision layout means that flooding will not cut roads in and out of the subdivision and isolate homes within the subdivision, although it will cut roads joining either side of the subdivision. This will minimise the risk of people entering high hazard flood waters to enter or leave their homes and evacuation of large areas would not be necessary;
- 5. Were individual homes to flood (and there is a low probability of this occurring) and people chose to evacuate (not necessary as most will be able to shelter in place) the scale of evacuation compared to the size of the subdivision and the short duration of the flooding means that staying with neighbours is a realistic and likely option.
- 6. It is unlikely that evacuation of these homes would interfere with flood evacuation from Windsor and McGraths Hill and, in my view, does not need to be considered.
- 7. Serious consideration should be given to designing future assets at the electrical substation so that they are well above the 1 in 500 level, perhaps even above the PMF given the space available on the block.

Should you require further clarification on any of the above matters please do not hesitate to contact me.
Yours faithfully
For Molino Stewart Pty Ltd

Steven Molino

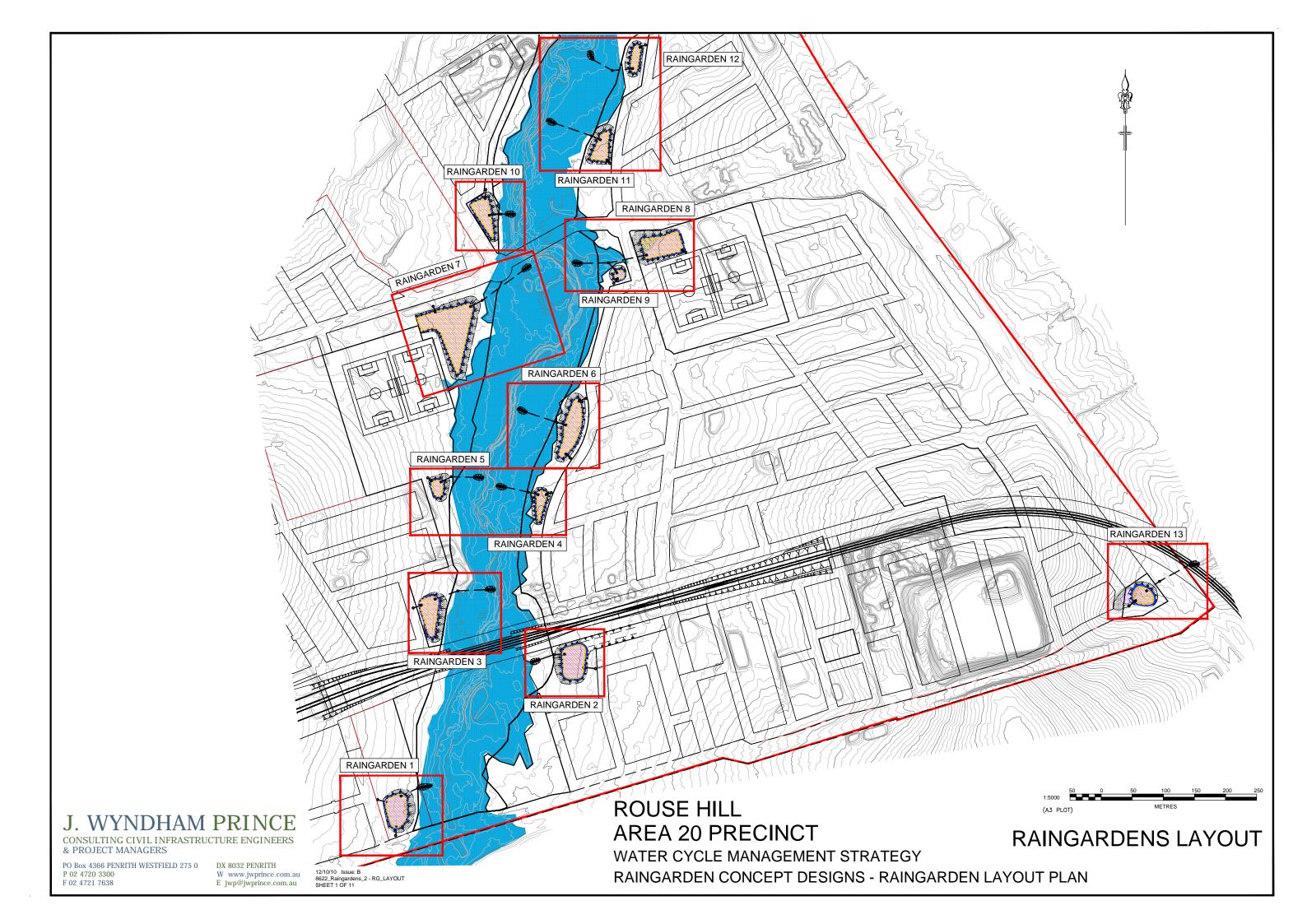
Principal

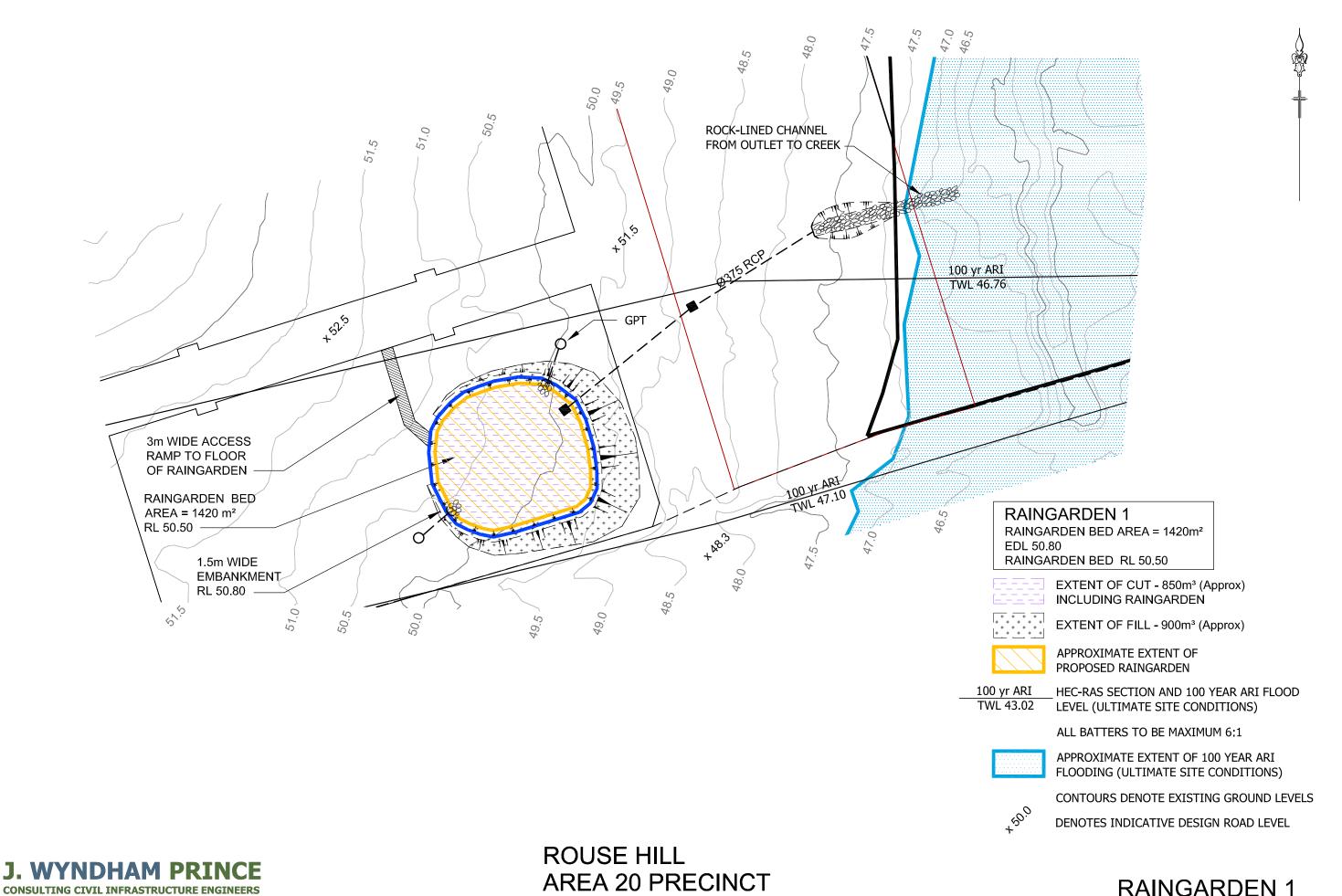
 $Y:\label{loss} \begin{tabular}{l} Y:\label{loss} \begin{tabular}{l} All Progress \end{tabular} \begin{tabular}{l$ 



## **Attachment I**

Detailed Concept Designs and Estimates of Quantities





## J. WYNDHAM PRINCE

PO Box 4366 PENRITH WESTFIELD 275 0 P 02 4720 3300 F 02 4721 7638

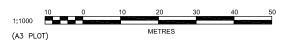
**& PROJECT MANAGERS** 

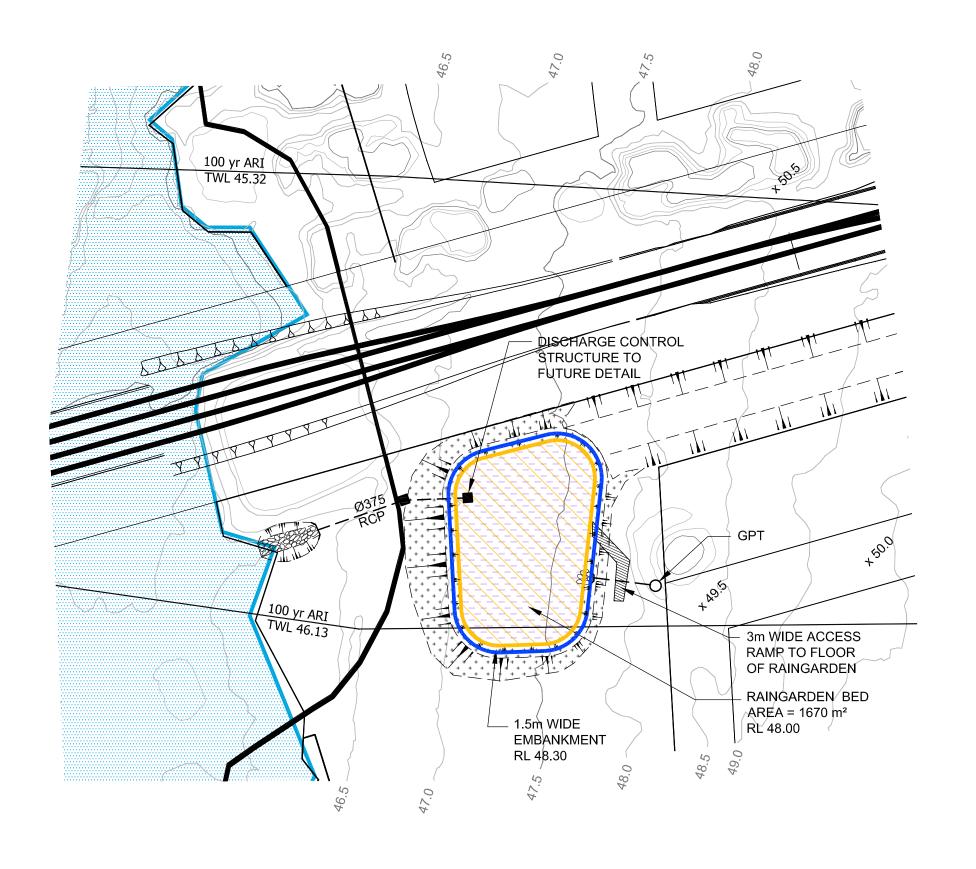
W www.jwprince.com.au E jwp@jwprince.com.au

22/10/10 Issue: B 8622\_Raingardens\_2 - RG\_1 SHEET 2 OF 11

# WATER CYCLE MANAGEMENT STRATEGY **RAINGARDEN CONCEPT DESIGNS - RAINGARDEN 1**

## **RAINGARDEN 1**







## **RAINGARDEN 2**

RAINGARDEN BED AREA =1670m<sup>2</sup> EDL 48.30 RAINGARDEN BED RL 48.00

EXTENT OF CUT -1170m³ (Approx) INCLUDING RAINGARDEN

+ + + + +

EXTENT OF FILL - 1030m³ (Approx)



APPROXIMATE EXTENT OF PROPOSED RAINGARDEN

100 yr ARI TWL 43.02 HEC-RAS SECTION AND 100 YEAR ARI FLOOD LEVEL (ULTIMATE SITE CONDITIONS)

ALL BATTERS TO BE MAXIMUM 6:1



APPROXIMATE EXTENT OF 100 YEAR ARI FLOODING (ULTIMATE SITE CONDITIONS)

CONTOURS DENOTE EXISTING GROUND LEVELS



DENOTES INDICATIVE DESIGN ROAD LEVEL

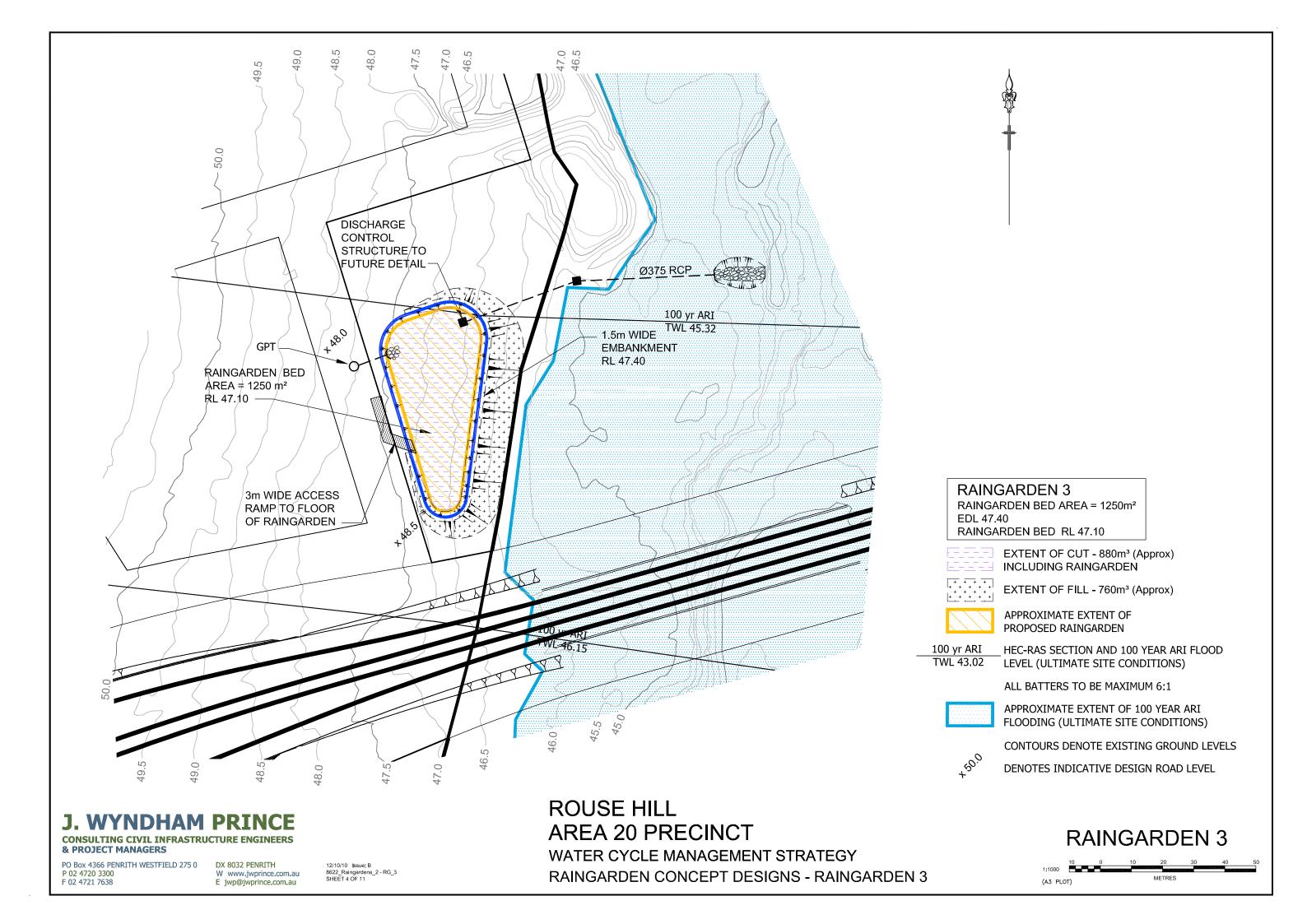
# J. WYNDHAM PRINCE CONSULTING CIVIL INFRASTRUCTURE ENGINEERS

& PROJECT MANAGERS

PO Box 4366 PENRITH WESTFIELD 275 0 P 02 4720 3300 F 02 4721 7638 DX 8032 PENRITH W www.jwprince.com.au E jwp@jwprince.com.au 12/10/10 Issue: B 8622\_Raingardens\_2 - RG\_2 SHEET 3 OF 11 ROUSE HILL
AREA 20 PRECINCT
WATER CYCLE MANAGEMENT STRATEGY
RAINGARDEN CONCEPT DESIGNS - RAINGARDEN 2

## **RAINGARDEN 2**





## **RAINGARDEN 5**

RAINGARDEN BED AREA = 400m<sup>2</sup> EDL 46.30 RAINGARDEN BED RL 46.00



EXTENT OF CUT - 300m³ (Approx) INCLUDING RAINGARDEN



EXTENT OF FILL - 350m3 (Approx)



RAINGARDEN BED AREA = 485m<sup>2</sup> EDL 45.50

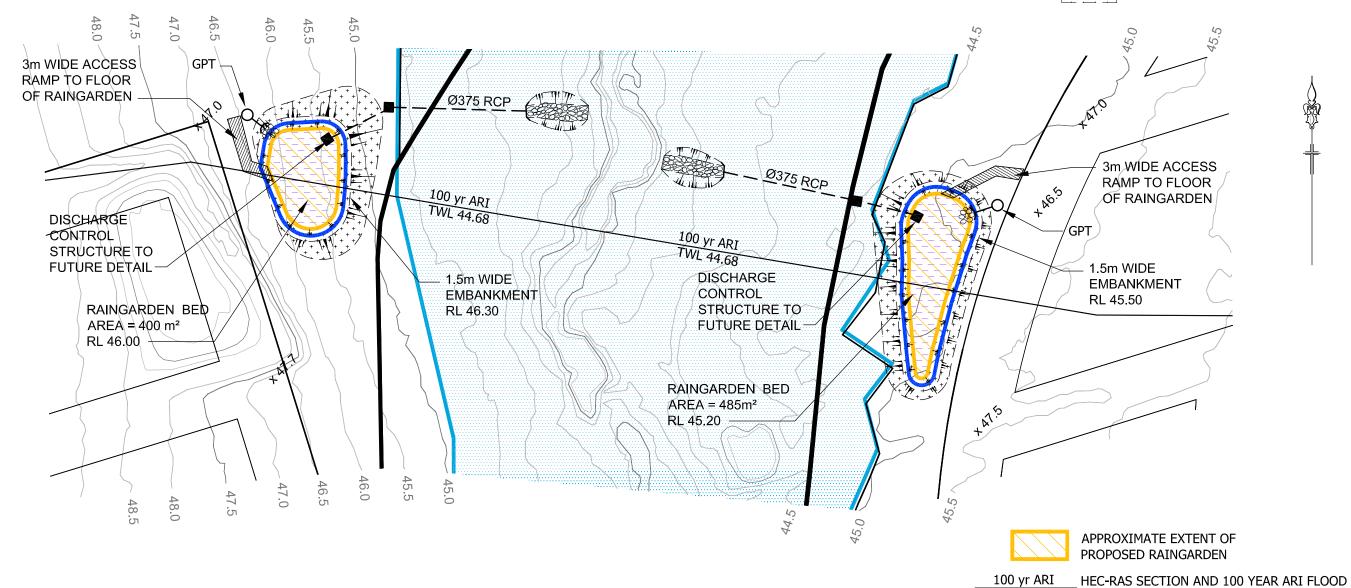
RAINGARDEN BED RL 45.20

EXTENT OF CUT - 390m³ (Approx) INCLUDING RAINGARDEN



TWL 43.02

EXTENT OF FILL - 380m³ (Approx)



J. WYNDHAM PRINCE

CONSULTING CIVIL INFRASTRUCTURE ENGINEERS & PROJECT MANAGERS

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WATER CYCLE MANAGEMENT STRATEGY
RAINGARDEN CONCEPT DESIGNS - RAINGARDENS 4 & 5

RAINGARDENS 4 & 5

DENOTES INDICATIVE DESIGN ROAD LEVEL

LEVEL (ULTIMATE SITE CONDITIONS)

APPROXIMATE EXTENT OF 100 YEAR ARI FLOODING (ULTIMATE SITE CONDITIONS)

CONTOURS DENOTE EXISTING GROUND LEVELS

ALL BATTERS TO BE MAXIMUM 6:1

